

Fig. 3. Hydro-topographic conditions of DeltaTelang I

*B. The Instruments of Mike-21 FM Model*

Erosion modeling process began with the collection of data as input. For bathymetric condition, a digitized ocean map and bathymetric measurement data were used. Configuration was then developed by setting up the mesh and bathymetric modeling.

The next stage was the preparation of input data for large hydrodynamic (HD) domain module (global) and spectral wave (SW) module using the medium domain. Data prepared for HD module included the boundary conditions in the form of tides data.

Data for SW module included heights and periods of significant waves from hind casting analysis and output data from the large HD in the form of water elevation data. Prior to using the large HD data for SW and small HD, calibration must be made using tide and flow data.

The next step was modeling the medium domain of SW module and small HD module. The following step was verification of tide data measurement in the study area and performing the analysis on the medium domain of SW module and small HD module. Subsequently, Sand Sediment Transport (ST) was modeled using small domain of HD module.

Operation and maintenance scenario consisted of 3 approaches as shown in Fig. 4, Fig. 5 and Fig. 6.

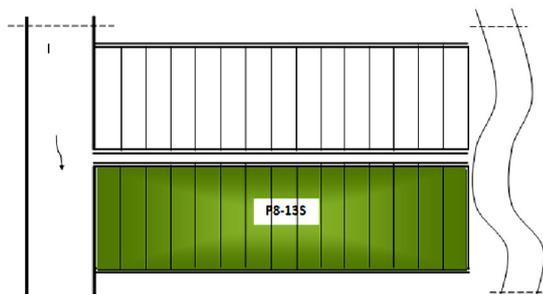


Fig. 4. Scenario I (O&M 25%)

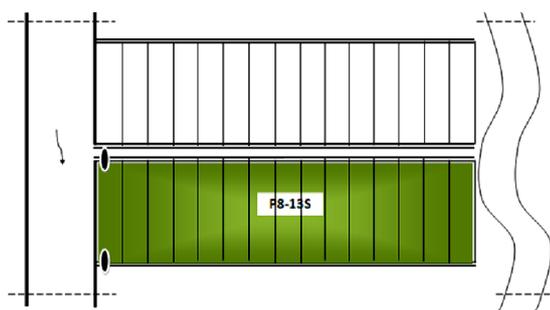


Fig. 5. Scenario II (O&M 50%)

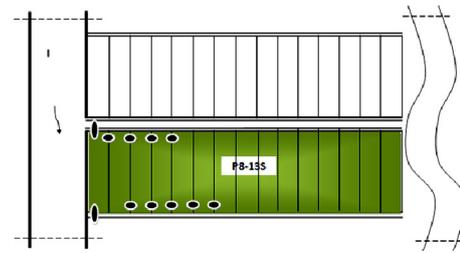


Fig. 6. Scenario III (O&M 75%):

III. DATA COLLECTION

*A. The Data*

Primary data were collected through field observation and direct measurement. In addition, other relevant data were also inventoried from the office of the relevant agencies in the form of secondary data.

Indicators of canal instability include internal factors and external factors. Internal factors such as canal hydraulic parameters are discharge and flow velocity and height of the tide and low tide, canal shape and dimensions (length, width, height of the water, as well as canal materials and diameter coefficient granules). External factors such as operation and maintenance activities, ship movement and water user participation were also collected.

*B. Surveys and Line Profile Measurement*

Survey of canal network used hydraulic simulation to determine the profile of the canals. In addition, the survey also collected information on canal structure such as structure type, hydraulic conditions, threshold peak, and crest length. From the above information, “n” Manning Coefficient was derived. This coefficient was then used to simulate sand transport model using softwares.

*C. Soil Data*

Soil samples were collected at several points on a secondary canal to represent different soil types along the canal. Disturbed soil samples from the canal were analyzed in soil mechanical laboratory using sieve to obtain grain diameters of  $d_{35}$ ,  $d_{50}$ , and  $d_{90}$ .

IV. RESULTS AND DISCUSSION

*A. Canal Profile*

The schematic profiles of primary and secondary canals and Based on the measurement, the dimensions of the secondary canal were 10 m in surface width, 2 m in bottom width, and 1.50 m in average depth .

All secondary canals were generally unstable, except SPD canals in P6 and P8 and SDU canals in P10 and P12.

*B. Sediment Materials Due to Erosion*

Sediment smaller than  $2\mu\text{m}$  (clay) is generally considered as cohesive sediment, whereas coarse sediment with the size greater than  $60\mu\text{m}$  is considered as non-cohesive sediment and the mud (silt) which size is 2 -  $60\mu\text{m}$  is considered among the cohesive and non-cohesive sediment [3].

Results indicated that grain size distributed in SPD canal has the average diameter of  $797\mu\text{m}$  which was considered as non-cohesive sediment. In SDU canal, the average grain size was  $793\mu\text{m}$  which was also considered as non-cohesive sediment. Grain size distribution in SPD canal is shown in

Fig. 7. and Grain size distribution in SDU canal is shown in Fig. 8.

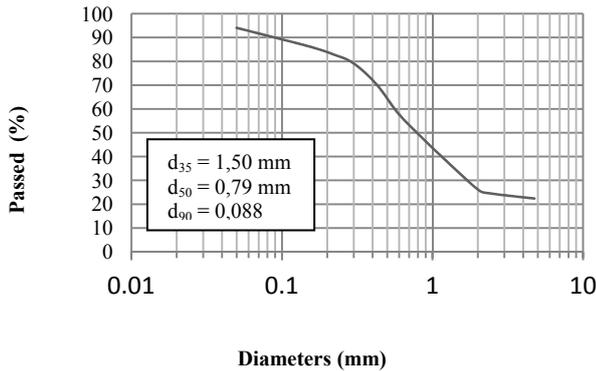


Fig. 7. Grain size distribution in SPD canal

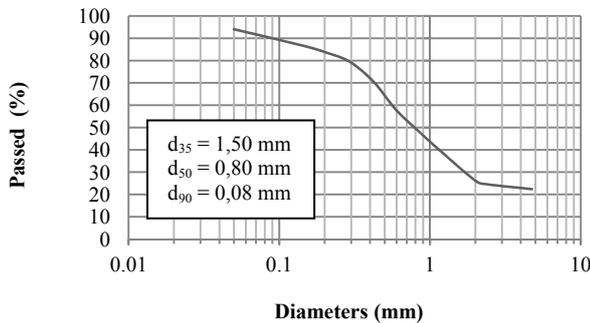


Fig. 8. Grain size distribution in SDU canal

### C. Implication on OM for Agriculture

Operation and maintenance (OM) of water infrastructures are intended towards fulfilment of crop water needs. Proper OM can be achieved if canal flow is in good capacity. This requires stable canal both in static and dynamic conditions.

In order to achieve stable canal, erosion must be controlled by considering canal dimensions and structures, proper operations of gates to obtain proper flow and the specific characteristics of tidal lowlands. [4]- [6].

## V. CONCLUSIONS AND RECOMMENDATIONS

### A. Conclusions

From this research, the following are concluded:

- 1) All secondary canals were generally unstable, except SPDs at canal section 6 (P6) and P8 and SDUs at P10 and P12.
- 2) Sediment grains in both SPDs and SDUs were categorized non-cohesive sediment. Erosion occurred in SPDs at P0, P2, P4 and P6 and in SDUs at P0, P2, and P4.

### B. Recommendations

Different scenarios should be made on the basis of n-Manning coefficient, sluice gates and flow velocity.

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